

MIC-S90-AH-L 2179502-06

Hilti North America Installation Technical Manual Technical Data MI System

Version 1.2 08.2017





Terms of common cooperation / Legal disclaimer

The product technical data published in these Technical Data Sheets are only valid for the mentioned codes or technical data generation methods and the defined application conditions (e.g. ambient temperature load capacity not valid in case of fire, data not valid in support structures when mixed with third party products, values only apply to static loading conditions). Technical data applies to the component only — suitability and capacity of all other components must be checked separately by the responsible engineer (e.g., other assembly components, attachments, base materials, and building structures).

Suitability of structures combining different products for specific applications needs to be verified by conducting a system design and calculation, using for example Hilti PROFIS software. In addition, it is crucial to fully respect the Instructions for Use and to assure clean, unaltered and undamaged state of all products at any time in order to achieve optimum performance (e.g. avoid misuse, modification, overload, corrosion).

As products but also technical data generation methodologies evolve over time, technical data might change at any time without prior notice. We recommend to use the latest technical data sheets published by Hilti.

In any case the suitability of structures combining different products for specific applications need to be checked and cleared by an expert, particularly with regard to compliance with applicable norms, codes, and project specific requirements, prior to using them for any specific facility. This book only serves as an aid to interpret the capacity of the components listed, without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application. User must take all necessary and reasonable steps to prevent or limit damage. The suitability of structures combining different products for specific applications need to be confirmed with a professional designer and/or structural engineers to ensure compliance with User's specific jurisdiction and project requirements.



Designation	Item number
MIC-S90-AH-500	2179502
MIC-S90-AH-750	2179503
MIC-S90-AH-1000	2179504
MIC-S90-AH-1500	2179505
MIC-S90-AH-2000	2179506

Corrosion protection:

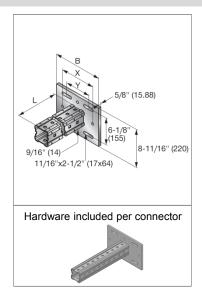
Hot dipped galvanized per ASTM A123

Girder - 2.95 mils ($75 \mu m$) minimum Plate - 3.94 mils ($100 \mu m$) minimum

Weight:

MIC-S90-AH-500	26.01 lb (11800g)
MIC-S90-AH-750	31.22 lb (14160g)
MIC-S90-AH-1000	36.40 lb (16510g)
MIC-S90-AH-1500	46.78 lb (21220g)
MIC-S90-AH-2000	57.17 lb (25930g)
B 1.41	

Description:



Hilti Hot-dipped galvanized bracket, typically used for fixation to a steel beam. The bracket is connected with beam clamps or using threaded rods through the slotted holes. Comes in different plate sizes to fit various steel beam sizes and in different lengths depending on application needs.

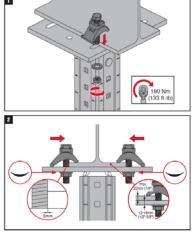
Material properties Material	Yield strength	Ultimate strength	E-modulus	Shear modulus
Bracket: ASTM A36 / A36M - 2014	$f_y = 36 \text{ ksi } (250 \frac{N}{mm^2})$	$f_u = 58 \text{ ksi } (400 \frac{N}{mm^2})$	29000 ksi (200000 $\frac{N}{mm^2}$)	11000 ksi (75845 $\frac{N}{mm^2}$)

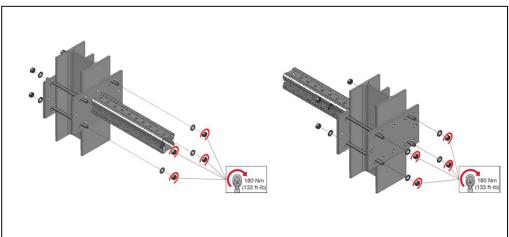
Instruction For Use:

No IFU attached to the package

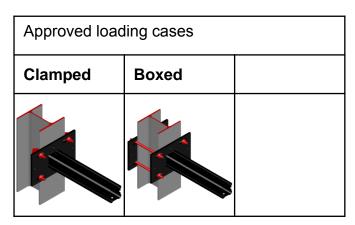
For clamped loading case

For boxed loading case (not attached to the packaging)









Governing Conditions

Methodology:

Connection strength values are determined with a combination of simulation (ANSYS®), calculation (Microsoft Excel and Mathcad) and testing.

Standards and codes:

ANSI/AISC 360-10 Specification for Structural Steel Buildings

ANSI/AISC 360-10— Inelastic analysis

Appendix 1

AISC Steel Design Column Base Plates

Guide Series 1

AISI S100 - 2007/2010 North American Specification for the Design of cold

formed Steel Structural Members

EN 1993-1-1 Eurocode 3: Design of steel structures – Part 1-1: 03.2012

General rules and rules for buildings

EN 1993-1-8 Eurocode 3: Design of steel structures – Part 1-8: 03.2012

Design of joints

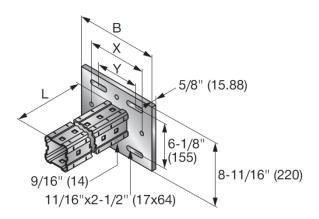
EN 10025-2 Hot rolled products of structural steels-Part 2: technical 02.2005

delivery conditions for non-alloy structural steels

Validity:

Temperature limits: -22°F (-30°C) to 200°F (+93°C).

Published allowable loads for applications are based on static loading conditions. Non-static forces, including those resulting from thermal or other expansion must be taken into account during design.





Clamped	Boxed	
To begress product the lates to the latest year that and what is to the latest to the latest to the latest to the latest		

Loading case: Clamped		Combinations covered by load	ding case
Bill of Material for this lo Bracket 1x MIC-S90-AH-500 MIC-S90-AH-750 MIC-S90-AH-1000 MIC-S90-AH-2000 Hardware not included in Beam clamps 4x MI-SGC M16	2179502 2179503 2179504 2179505 2179506	Bracket used for a perpendicular connection to flange of structural steel profiles. For flange width 2.95" (75mm) -6.47" (165mm).	The long Sound an Alenda Sound like on the binang page halde deep technologies. 1. May been from proof the part Soundaries. 2. May been dischool into paper 10 paper 100 particum et = \frac{2}{2} decrease \$\text{NC} \text{TW} \text{2.50}} interference \$\text{NC} \text{2.50}} interference \$\text{2.50}} interference \$\text{2.50} interference \$\text{2.50}} interference \$\text{2.50}} interference \$\text{2.50}} interference \$\text{2.50}} interference \$

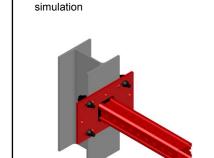
Usage of Values for Design Strength and Allowable Strength

The Design Strength and Allowable Strength tables on the following pages include strength reduction factors:

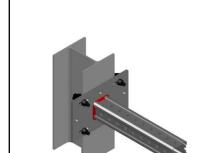
- ASD: Safety Factor (omega) > 1.0 as per AISC specifications.
- **LRFD:** Strength Reduction Factor (phi) < 1.0 as per AISC specifications. $\Omega = \frac{1.5}{\phi}$ (Reference AISC 360 C-B3-5)

Factored loads are required for input to the given interaction equations. Factored loads are the responsibility of the user. Factored loads are noted as P, V and M

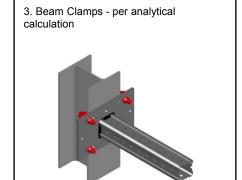
Limiting components of capacity evaluated in following tables:



1. Bracket per FEA

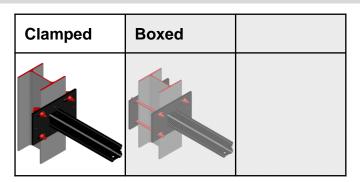


2. Welds - per analytical calculation



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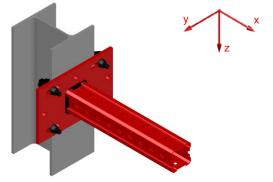


Values for Design Strength and Allowable Strength

1/3

NOTE: Calculate interaction separately for each group only using values from that group. Limiter is defined by highest interaction. Use absolute values. Values refer to the coordinate system shown.

1. Bracket per FEA simulation



	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
LRFD*	21.92	23.31	11.51	11.51	11.51	11.51
LKFD	+Mx [kip*ft]	-Mx [kip*ft]	+My [kip*ft]	-My [kip*ft]	+Mz [kip*ft]	-Mz [kip*ft]
	3.02	3.02	4.19	4.19	4.19	4.19

	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
A C D*	14.58	15.51	7.66	7.66	7.66	7.66
ASD*	+Mx	-Mx	+My	-My	+Mz	-Mz
	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]
	2.01	2.01	2.79	2.79	2.79	2.79

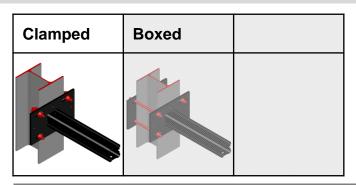
Interaction for LRFD

$$\frac{P_{ux}}{F_{x}} + \frac{V_{uy}}{F_{y}} + \frac{V_{uz}}{F_{z}} + \frac{M_{ux}}{M_{x}} + \frac{M_{uy}}{M_{y}} + \frac{M_{uz}}{M_{z}} \le 1$$

Interaction for ASD:

$$\frac{P_{ax}}{F_{x}} + \frac{V_{av}}{F_{y}} + \frac{V_{az}}{F_{z}} + \frac{M_{ax}}{M_{x}} + \frac{M_{av}}{M_{y}} + \frac{M_{az}}{M_{z}} \le 1$$

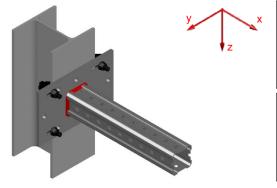




Values for Design Strength and Allowable Strength

NOTE: Calculate interaction separately for each group only using values from that group. Limiter is defined by highest interaction. Use absolute values. Values refer to the coordinate system shown.

2. Welds - per analytical calculation



	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
1 DED*	58.00	58.00	27.34	27.34	27.34	27.34
LRFD*	+Mx	-Mx	+My	-My	+Mz	-Mz
	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]
	7.36	7.36	4.56	4.56	4.56	4.56
	, Ev	E.	15	Ev.		F-
	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	[Lim]	[Lim]	[Lim]	[Lim]	[Lim]	[leim]

	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
ASD*	38.67	38.67	18.23	18.23	18.23	18.23
ASD	+Mx	-Mx	+My	-My	+Mz	-Mz
	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]
	4.90	4.90	3.04	3.04	3.04	3.04

Interaction for LRFD
$$\frac{P_{ux}}{F_x} + \frac{V_{uy}}{F_y} + \frac{V_{uz}}{F_z} + \frac{M_{ux}}{M_x} + \frac{M_{uy}}{M_y} + \frac{M_{uz}}{M_z} \le 1$$

Interaction for ASD:

$$\frac{P_{ax}}{F_{x}} + \frac{V_{ay}}{F_{y}} + \frac{V_{az}}{F_{z}} + \frac{M_{ax}}{M_{x}} + \frac{M_{ay}}{M_{y}} + \frac{M_{az}}{M_{z}} \le 1$$

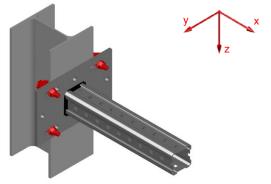


Clamped	Boxed	

Values for Design Strength and Allowable Strength

NOTE: Calculate interaction separately for each group only using values from that group. Limiter is defined by highest interaction. Use absolute values. Values refer to the coordinate system shown.

3. Beam Clamps - per analytical calculation



	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
LRFD*	21.58	Not decisive	2.32	2.32	2.32	2.32
21 11 2	+Mx	-Mx	+My	-My	+Mz	-Mz
	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]
	0.62	0.62	5.10	5.10	2.94	2.94
	+Fx	-Fx	+Fy	-Fv	+Fz	-Fz
	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
∆SD*	[kip] 14.39	[kip] Not decisive	[kip] 1.55	[kip] 1.55	[kip] 1.55	[kip] 1.55
ASD*		Not				
ASD*	14.39	Not decisive	1.55	1.55	1.55	1.55

Interaction for LRFD

Normal force interaction:

$$\frac{\overline{P_{ux}}}{F_x} + \frac{M_{uy}}{M_y} + \frac{M_{uz}}{M_z} \le 1$$

Shear force interaction:

- Shear Interaction Equation is only valid for TENSILE P_{ux} loads ($P_{ux} > 0$). Equation is not valid for
- compressive P_{ux} loads ($P_{ux} < 0$). For Shear interaction, user must ADDITIONALLY verify: $P_{ux} / F_x < 1$.

$$\sqrt{\left(\frac{V_{uy}}{F_y \times \left(1 - \frac{P_{ux}}{F_x}\right)}\right)^2 + \left(\frac{V_{uz}}{F_z \times \left(1 - \frac{P_{ux}}{F_x}\right)}\right)^2} + \frac{M_{ux}}{M_x \times \left(1 - \frac{P_{ux}}{F_x}\right)} \le 1$$

Interaction for ASD:

Normal force interaction:

$$\frac{P_{ax}}{F_x} + \frac{M_{ay}}{M_y} + \frac{M_{az}}{M_z} \le 1$$

Shear force interaction:

- Shear Interaction Equation is \underline{only} valid for TENSILE P_{ax} loads ($P_{ax} > 0$). Equation is \underline{not} valid for compressive P_{ax} loads ($P_{ax} < 0$).

 For Shear interaction, user must ADDITIONALLY verify: $P_{ax} / P_x < 1$.

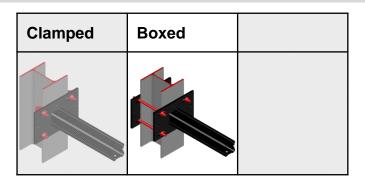
$$\sqrt{\left(\frac{V_{ay}}{F_y \times \left(1 - \frac{P_{ax}}{F_x}\right)}\right)^2 + \left(\frac{V_{az}}{F_z \times \left(1 - \frac{P_{ax}}{F_x}\right)}\right)^2 + \frac{M_{ax}}{M_x \times \left(1 - \frac{P_{ax}}{F_x}\right)}} \le 1$$



Nut

8x M16-F nut

MIC-S90-AH-L Bracket - Steel



Loading case: Boxed Combinations covered by loading case Bill of Material for this loading case: Bracket used for a perpendicular connection **Bracket** 1x MIC-S90-AH-500 2179502 of MI-90 girder to flange MIC-S90-AH-750 2179503 of structural steel profiles. MIC-S90-AH-1000 2179504 For flange width 2.95" (75mm) - 6.47" (165mm). MIC-S90-AH-1500 2179505 MIC-S90-AH-2000 2179506 Hardware not included in packaging: Base plate 1x MIB-SAH 2174674 Threaded rods cut to particular length 4x AM16x1000 8.8 HDG...m 419104 Lock washer 8x LW M16 HDG plus washer 2185343

Usage of Values for Design Strength and Allowable Strength

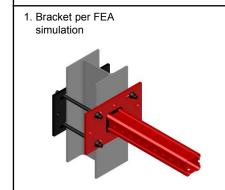
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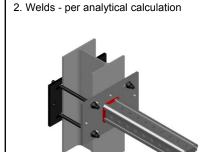
The Design Strength and Allowable Strength tables on the following pages include strength reduction factors:

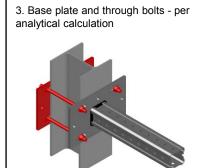
- ASD: Safety Factor (omega) > 1.0 as per AISC specifications.
- **LRFD:** Strength Reduction Factor (phi) < 1.0 as per AISC specifications. $\Omega = \frac{1.5}{6}$ (Reference AISC 360 C-B3-5)

Factored loads are required for input to the given interaction equations. Factored loads are the responsibility of the user. Factored loads are noted as P, V and M

Limiting components of capacity evaluated in following tables:

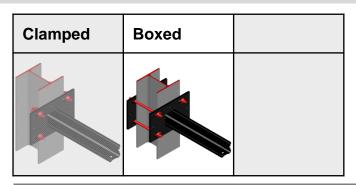






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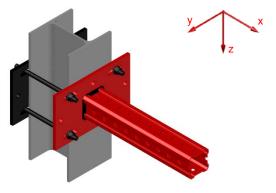




Values for Design Strength and Allowable Strength

NOTE: Calculate interaction separately for each group only using values from that group. Limiter is defined by highest interaction. Use absolute values. Values refer to the coordinate system shown.

1. Bracket per FEA simulation



	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
LRFD*	22.62	23.31	11.51	11.51	11.51	11.51
LKFD	+Mx [kip*ft]	-Mx [kip*ft]	+My [kip*ft]	-My [kip*ft]	+Mz [kip*ft]	-Mz [kip*ft]
	3.02	3.02	4.19	4.19	4.19	4.19

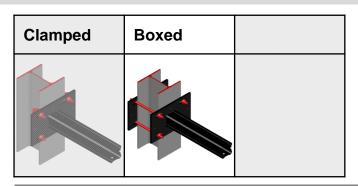
ASD*	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
	15.05	15.51	7.66	7.66	7.66	7.66
	+Mx	-Mx	+My	-My	+Mz	-Mz
	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]
	2.01	2.01	2.79	2.79	2.79	2.79

Interaction for LRFD

$$\frac{P_{ux}}{F_x} + \frac{V_{uv}}{F_y} + \frac{V_{uz}}{F_z} + \frac{M_{ux}}{M_x} + \frac{M_{uv}}{M_y} + \frac{M_{uz}}{M_z} \le 1$$

$$\frac{P_{ax}}{F_x} + \frac{V_{av}}{F_y} + \frac{V_{az}}{F_z} + \frac{M_{ax}}{M_x} + \frac{M_{av}}{M_y} + \frac{M_{az}}{M_z} \le 1$$

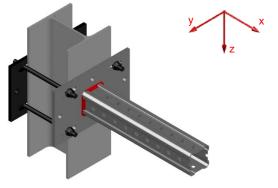




Values for Design Strength and Allowable Strength

NOTE: Calculate interaction separately for each group only using values from that group. Limiter is defined by highest interaction. Use absolute values. Values refer to the coordinate system shown.

2. Welds - per analytical calculation



	+Fx	-Fx	+Fv	-Fv	+Fz	-Fz
LRFD*	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
	58.00	58.00	27.34	27.34	27.34	27.34
	+Mx	-Mx	+My	-My	+Mz	-Mz
	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]
	7.36	7.36	4.56	4.56	4.56	4.56
	+Fx	-Fx	+Fv	-Fv	+F7	-F7

ASD*	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	[kip]	[kip]	[kip]	[kip]	[kip]	[kip]
	38.67	38.67	18.23	18.23	18.23	18.23
	+Mx	-Mx	+My	-My	+Mz	-Mz
	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]
	4.90	4.90	3.04	3.04	3.04	3.04

Interaction for LRFD
$$\frac{P_{ux}}{F_x} + \frac{V_{uv}}{F_y} + \frac{V_{uz}}{F_z} + \frac{M_{ux}}{M_x} + \frac{M_{uv}}{M_y} + \frac{M_{uz}}{M_z} \le 1$$

$$\frac{P_{ax}}{F_{x}} + \frac{V_{ay}}{F_{y}} + \frac{V_{az}}{F_{z}} + \frac{M_{ax}}{M_{x}} + \frac{M_{ay}}{M_{y}} + \frac{M_{az}}{M_{z}} \le 1$$

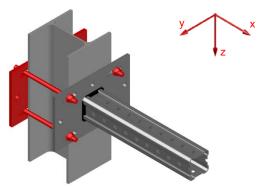


Clamped	Boxed	

Values for Design Strength and Allowable Strength

NOTE: Calculate interaction separately for each group only using values from that group. Limiter is defined by highest interaction. Use absolute values. Values refer to the coordinate system shown.

3. Base plate and through bolts - per analytical calculation



ulation						
LRFD*	+Fx [kip]	-Fx [kip]	+Fy [kip]	-Fy [kip]	+Fz [kip]	-Fz [kip]
	34.60	Not decisive	7.27	7.27	7.27	7.27
	+Mx [kip*ft]	-Mx [kip*ft]	+My [kip*ft]	-My [kip*ft]	+Mz [kip*ft]	-Mz [kip*ft]
	1.93	1.93	8.06	8.06	4.71	4.71
ASD*	+Fx [kip]	-Fx [kip]	+Fy [kip]	-Fy [kip]	+Fz [kip]	-Fz [kip]
	23.07	Not decisive	4.84	4.84	4.84	4.84
	+Mx [kip*ft]	-Mx [kip*ft]	+My [kip*ft]	-My [kip*ft]	+Mz [kip*ft]	-Mz [kip*ft]
	1.29	1.29	5.37	5.37	3.14	3.14

Interaction for LRFD

Normal force interaction:

$$\frac{P_{ux}}{F_x} + \frac{M_{uy}}{M_y} + \frac{M_{uz}}{M_z} \le 1$$

Shear force interaction:

- Shear Interaction Equation is \underline{only} valid for TENSILE P_{ux} loads ($P_{ux} > 0$). Equation is \underline{not} valid for
- compressive P_{ux} loads ($P_{ux} < 0$). For Shear interaction, user must ADDITIONALLY verify: P_{ux} / $F_x < 1$.

$$\sqrt{\left(\frac{V_{uy}}{F_y \times \left(1 - \frac{P_{ux}}{F_x}\right)}\right)^2 + \left(\frac{V_{uz}}{F_z \times \left(1 - \frac{P_{ux}}{F_x}\right)}\right)^2} + \frac{M_{ux}}{M_x \times \left(1 - \frac{P_{ux}}{F_x}\right)} \le 1$$

Interaction for ASD:

Normal force interaction:

$$\frac{P_{ax}}{F_x} + \frac{M_{ay}}{M_y} + \frac{M_{az}}{M_z} \le 1$$

Shear force interaction:

- Shear Interaction Equation is <u>only</u> valid for TENSILE P_{ax} loads ($P_{ax} > 0$). Equation is <u>not</u> valid for compressive P_{ax} loads ($P_{ax} < 0$). - For Shear interaction, user must ADDITIONALLY verify: P_{ax} / $F_{x} < 1$.

$$\sqrt{\left(\frac{V_{ay}}{F_y \times \left(1 - \frac{P_{ax}}{F_x}\right)}\right)^2 + \left(\frac{V_{az}}{F_z \times \left(1 - \frac{P_{ax}}{F_x}\right)}\right)^2} + \frac{M_{ax}}{M_x \times \left(1 - \frac{P_{ax}}{F_x}\right)} \le 1$$





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en español: 1-800-879-5000 Fax: 1-800-879-7000

www.us.hilti.com

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Fax: 1-800-363-4459

www.hilti.ca

The data contained in this literature was current as of the date of publication. Updates and changes may be made based on later testing. If verification is needed that the data is still current, please contact the Hilti Technical Support Specialists at 1-800-879-8000 (U.S.) or 1-800-363-4458 (Canada). All published load values contained in this literature represent the result of testing by Hilti or test organizations. Local base materials were used. Because of variations in materials, on-site testing is necessary to determinate performance at any specific site.