

# MIC-S90-AA 304811

Hilti North America Installation Technical Manual Technical Data MI System

Version 1.2 08.2017



# Terms of common cooperation / Legal disclaimer

The product technical data published in these Technical Data Sheets are only valid for the mentioned codes or technical data generation methods and the defined application conditions (e.g. ambient temperature load capacity not valid in case of fire, data not valid in support structures when mixed with third party products, values only apply to static loading conditions). Technical data applies to the component only -- suitability and capacity of all other components must be checked separately by the responsible engineer (e.g., other assembly components, attachments, base materials, and building structures).

Suitability of structures combining different products for specific applications needs to be verified by conducting a system design and calculation, using for example Hilti PROFIS software. In addition, it is crucial to fully respect the Instructions for Use and to assure clean, unaltered and undamaged state of all products at any time in order to achieve optimum performance (e.g. avoid misuse, modification, overload, corrosion).

As products but also technical data generation methodologies evolve over time, technical data might change at any time without prior notice. We recommend to use the latest technical data sheets published by Hilti.

In any case the suitability of structures combining different products for specific applications need to be checked and cleared by an expert, particularly with regard to compliance with applicable norms, codes, and project specific requirements, prior to using them for any specific facility. This book only serves as an aid to interpret the capacity of the components listed, without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application. User must take all necessary and reasonable steps to prevent or limit damage. The suitability of structures combining different products for specific applications need to be confirmed with a professional designer and/or structural engineers to ensure compliance with User's specific jurisdiction and project requirements.



MIC-S90-AA 304811	Designation	Item number
	MIC-S90-AA	304811

# **Corrosion protection:**

 Hot dipped galvanized per DIN EN ISO 1461:

 Connector:
 2.2 mils (55 μm)

 Bolt:
 1.8 mils (45 μm)

 Nut:
 1.8 mils (45 μm)

#### Weight:

9.63 lb (4370 g) incl. components

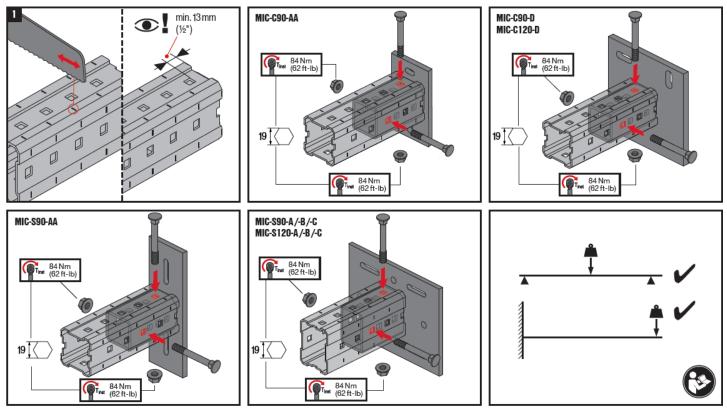
#### **Description:**

Hilti Hot-dipped galvanized baseplate connector, used for connecting a MI-90 girder to a steel beam using M12 mounting hardware. Two slotted holes enable fine tuning of baseplate position, and girder is connected using beam clamps or threaded rod. Comes in different plate sizes to fit various steel beam sizes.

1/2" (12) 2-3/8"x1/2" (60x13) MI-90 3-15/16" (300)
Hardware included per connector

Material properties				
Material	Yield strength	Ultimate strength	E-modulus	Shear modulus
Connector:				
S235JR - DIN EN10025-2 2005.4	f <sub>y</sub> = 34.08 ksi (235 $\frac{N}{mm^2}$ )	f <sub>u</sub> = 52.21 ksi (360 $\frac{N}{mm^2}$ )	29000 ksi (200000 <sup>N</sup> / <sub>mm<sup>2</sup></sub> )	11000 ksi (75845 <u></u> )
One hand screw, prevail torque hex nut	- 111	mm	mm	nin
Class 8.8 - DIN EN 1993-1-8	f <sub>y</sub> = 92.82 ksi (640 $\frac{N}{mm^2}$ )	f <sub>u</sub> = 116.03 ksi (800 <sup>N</sup> / <sub>mm<sup>2</sup></sub> )	<b>29000 ksi</b> (200000 <u>mm</u> <sup>2</sup> )	11000 ksi (75845 <u></u> )
	$m_{mm^2}$	$m_{\rm u}^2 = 1.0000  {\rm Mor}  (0000  {\rm mm^2})$	20000 Nor (200000 mm <sup>2</sup> )	$mm^{2}$

### **Instruction For Use:**



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Approved load	ling cases	
Clamped	Boxed	

# **Governing Conditions**

# Methodology:

Connection strength values are determined with a combination of simulation (ANSYS<sup>®</sup>), calculation (Microsoft Excel and Mathcad) and testing.

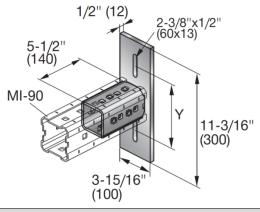
### Standards and codes:

<ul> <li>ANSI/AISC 360-10</li> </ul>	Specification for Structural Steel Buildings	
<ul> <li>ANSI/AISC 360-10–</li> </ul>	Inelastic analysis	
Appendix 1		
<ul> <li>AISC Steel Design</li> </ul>	Column Base Plates	
Guide Series 1		
<ul> <li>AISI S100 - 2007/2010</li> </ul>	North American Specification for the Design of cold	
	formed Steel Structural Members	
<ul> <li>ACI 318-08/11</li> </ul>	Building Code Requirement for Structural Concrete	
<ul> <li>EN 1993-1-1</li> </ul>	Eurocode 3: Design of steel structures – Part 1-1:	03.2012
	General rules and rules for buildings	
<ul> <li>EN 1993-1-8</li> </ul>	Eurocode 3: Design of steel structures – Part 1-8:	03.2012
	Design of joints	
• EN 10025-2	Hot rolled products of structural steels-Part 2: technical	02.2005
	delivery conditions for non-alloy structural steels	

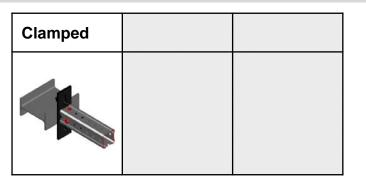
#### Validity:

Temperature limits:  $-22^{\circ}F(-30^{\circ}C)$  to  $200^{\circ}F(+93^{\circ}C)$ .

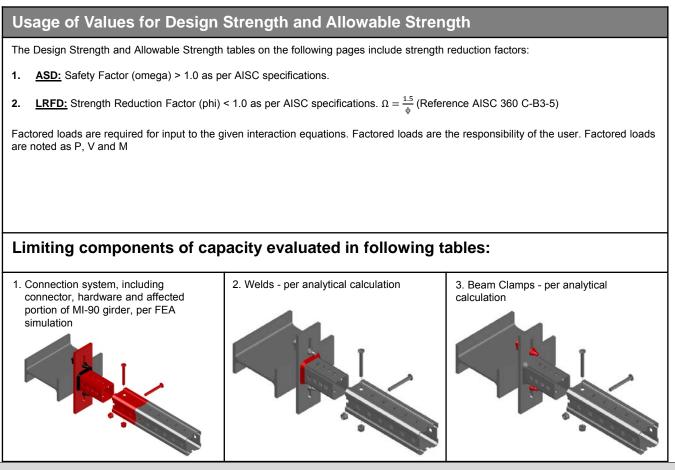
Published allowable loads for applications are based on static loading conditions. Non-static forces, including those resulting from thermal or other expansion must be taken into account during design.







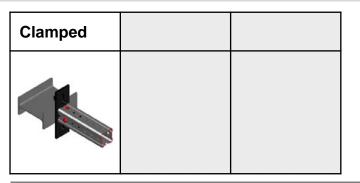
Loading case: Clamped	Combinations covered by loading case
Bill of Material for this loading case:         1x MIC-S90-AA       304811         Hardware not included in packaging:         Beam clamps         2x MI-SGC M12       233859	Connector used for a perpendicular connection of MI-90 girder to flange of structural steel profiles. For flange width 2.95" (75mm) -6.47" (165mm).



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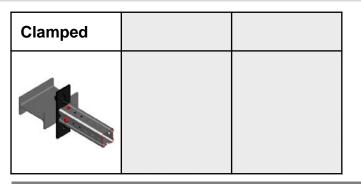
# Values for Design Strength and Allowable Strength

1/3

**NOTE**: Calculate interaction separately for each group only using values from that group. Limiter is defined by highest interaction. Use absolute values. Values refer to the coordinate system shown.

1. Connection system, including connector, hardw	are and affe	cted porti	ion of MI-9	90 girder,	per FEA s	simulation	ı
y z x	LRFD*	+Fx [kip] 4.92 +Mx [kip*ft] 2.10	-Fx [kip] 14.22 -Mx [kip*ft] 2.10	+Fy [kip] 5.84 +My [kip*ft] 1.40	-Fy [kip] 5.84 -My [kip*ft] 1.40	+Fz [kip] 7.64 +Mz [kip*ft] 1.34	-Fz [kip] 7.64 -Mz [kip*ft] 1.34
	ASD* Interactio $\frac{P_{ux}}{F_x} + \frac{V_{ux}}{F_x}$			+Fy [kip] 3.9 +My [kip*ft] 0.93 + $\frac{M_{uy}}{M}$ +	$-Fy$ [kip] 3.9 -My [kip*ft] 0.93 $-\frac{M_{uz}}{M} \leq -\frac{M_{uz}}{M}$	+Fz [kip] 5.09 +Mz [kip*ft] 0.89	-Fz [kip] 5.09 -Mz [kip*ft] 0.89
	<sup>x</sup> Interactio			У	Z		
	$\frac{P_{ax}}{F_x} + \frac{V_{ax}}{F_x}$	$\frac{V_{av}}{F_y} + \frac{V_a}{F_z}$	$\frac{z}{z} + \frac{M_{ax}}{M_x}$	$+\frac{M_{av}}{M_y}+$	$\frac{M_{az}}{M_z} \le 7$	1	
	*Values al in accorda			•			) safety (Ω ry.





# Values for Design Strength and Allowable Strength

2/3

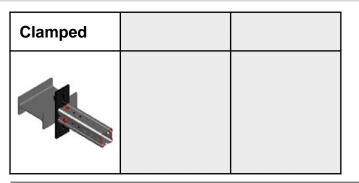
NOTE: Calculate interaction separately for each group only using values from that group. Limiter is defined by highest interaction. Use absolute values. Values refer to the coordinate system shown.

#### 2. Welds - per analytical calculation

y x		+Fx [kip]	-Fx [kip]	+Fy [kip]	-Fy [kip]	+Fz [kip]	-Fz [kip]	
		49.72	49.72	11.30	11.30	11.30	11.30	
Z	LRFD*	+Mx	-Mx	+My	-My	+Mz	-Mz	
		[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	
		4.61	4.61	2.45	2.45	2.45	2.45	
		+Fx	-Fx	+Fy	-Fy	+Fz	-Fz	
🔉 î		[kip]	[kip]	[kip]	[kip]	[kip]	[kip]	
	ASD*	33.14	33.14	7.53	7.53	7.53	7.53	
	AGD	+Mx	-Mx	+My	-My	+Mz	-Mz	
		[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	[kip*ft]	
		3.08	3.08	1.63	1.63	1.63	1.63	
	Interactio	n for LRF	D					
	$\frac{P_{ux}}{F_x} + \frac{V}{R}$	$\frac{V_{uv}}{F_y} + \frac{V_{uz}}{F_z}$	$\frac{X}{M} + \frac{M_{ux}}{M_x}$	$+\frac{M_{uv}}{M_y}+$	$-\frac{M_{uz}}{M_z} \le C$	1		
	Interaction for ASD:							
	$\frac{P_{ax}}{F_x} + \frac{V_{ax}}{F_x}$	$\frac{V_{av}}{F_y} + \frac{V_{az}}{F_z}$	$\frac{M_{ax}}{M_x}$	$+\frac{M_{av}}{M_{y}}+$	$-\frac{M_{az}}{M_z} \le 2$	1		

\*Values already include LRFD strength reduction ( $\Phi$ ) or ASD safety ( $\Omega$ ) factors in accordance with AISC, and are based on nominal geometry.





# Values for Design Strength and Allowable Strength

NOTE: Calculate interaction separately for each group only using values from that group. Limiter is defined by highest interaction. Use absolute values. Values refer to the coordinate system shown.

# 3. Beam Clamps - per analytical calculation

	+Fx [kip]	-Fx [kip]	+Fy [kip]	-Fy [kip]	+Fz [kip]	-Fz [kip]
LRFD*	3.52	Not decisive	0.74	0.74	0.74	0.74
	+Mx [kip*ft]	-Mx [kip*ft]	+My [kip*ft]	-My [kip*ft]	+Mz [kip*ft]	-Mz [kip*ft]
	0.13	0.13	0.60	0.60	0.52	0.52
	+Fx	-Fx	+Fy	-Fy	+Fz	-Fz
	+Fx [kip]	-Fx [kip]	+Fy [kip]	-Fy [kip]	+Fz [kip]	-Fz [kip]
<u> 490*</u>				,		
ASD*	[kip]	[kip] Not	[kip]	[kip]	[kip]	[kip]

# Interaction for LRFD

#### Normal force interaction:

The eccentricity ey and ez between the point of force transfer channel / connector and baseplate, which generates an additional bending moment on the system , must be taken into account in the interaction formula.

$$\frac{P_{ux}}{F_x} + \frac{V_{uy} * ey}{M_z} + \frac{V_{uz} * ez}{M_y} + \frac{M_{uy}}{M_y} + \frac{M_{uz}}{M_z} \le 1$$
Shear force interaction:
with e<sub>y</sub>=e<sub>z</sub>=0.070 m

#### Shear force interaction:

- Shear Interaction Equation is only valid for TENSILE Pux loads (Pux > 0). Equation is not valid for compressive  $P_{ux}$  loads ( $P_{ux} < \overline{0}$ ).

- For Shear interaction, user must ADDITIONALLY verify: 
$$P_{ux} / F_x < 1$$
.

$$\sqrt{\left(\frac{V_{uy}}{F_y \times \left(1 - \frac{P_{ux}}{F_x}\right)}\right)^2 + \left(\frac{V_{uz}}{F_z \times \left(1 - \frac{P_{ux}}{F_x}\right)}\right)^2 + \frac{M_{ux}}{M_x \times \left(1 - \frac{P_{ux}}{F_x}\right)} \le 1$$

Interaction for ASD:

#### Normal force interaction:

The eccentricity ey and ez between the point of force transfer channel / connector and baseplate, which generates an additional bending moment on the system , must be taken into account in the interaction formula.

$$\frac{P_{ax}}{F_x} + \frac{V_{av} * ey}{M_z} + \frac{V_{az} * ez}{M_y} + \frac{M_{av}}{M_y} + \frac{M_{az}}{M_z} \le 1 \qquad \text{with } e_y = e_z = 0.070 \text{ m}$$

#### Shear force interaction:

- Shear Interaction Equation is only valid for TENSILE Pax loads (Pax > 0). Equation is not valid for

compressive  $P_{ax}$  loads ( $P_{ax} < 0$ ). - For Shear interaction, user must ADDITIONALLY verify:  $P_{ax} / F_x < 1$ .

$$\sqrt{\left(\frac{V_{ay}}{F_y \times \left(1 - \frac{P_{ax}}{F_x}\right)}\right)^2 + \left(\frac{V_{az}}{F_z \times \left(1 - \frac{P_{ax}}{F_x}\right)}\right)^2} + \frac{M_{ax}}{M_x \times \left(1 - \frac{P_{ax}}{F_x}\right)} \le 1$$

\*Values already include LRFD strength reduction ( $\Phi$ ) or ASD safety ( $\Omega$ ) factors in accordance with AISC, and are based on nominal geometry.



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